

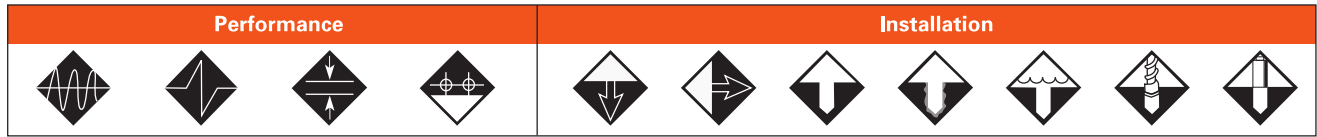
EPCON G5

Rebar (FE460)



1/8

Epoxy Resin - High Performance



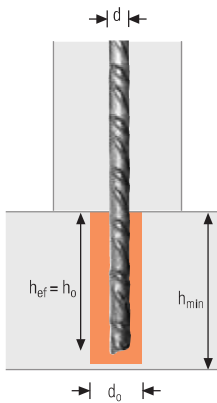
Technical Data



ICC-ES EVALUATION REPORT

EPCON G5		T8	T10	T12	T13	T16	T20	T25	T28	T32	T40
Dowel depth (mm)	$h_{ef,min}$	80	90	110	110	125	170	210	270	300	400
Ø bar (mm)	d	8	10	12	13	16	20	25	28	32	40
Ø drill bit (mm)	d_o	12	13	15	16	20	25	30	35	40	50
Drill depth (mm)	h_o	80	90	110	110	125	170	210	270	300	400
Min thick of base material (mm)	h_{min}	100	113	138	138	156	213	263	338	375	500
Ramset power tool code		DD543	DD543	DD543	DD543	DD565	DD565	DD565	DD565	DD576	DD576
Drill bit type-size		R3 PLUS-12	R3 PLUS-13	R3 PLUS-16	R3 PLUS-16	R3 MAX-20	R3 MAX-25	R3 MAX-30	R3 MAX-35	R3 MAX-40	R3 MAX-50

EPCON G5 Two part cartridge, 100% epoxy resin - vol. 650ml



Anchor Mechanical Properties

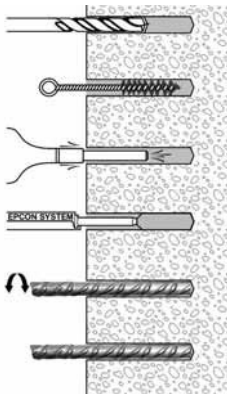
Rebar FE460	T8	T10	T12	T13	T16	T20	T25	T28	T32	T40
f_{yk} (N/mm ²) Yield strength	460	460	460	460	460	460	460	460	460	460
A_s (mm ²) Stressed cross-section	50.3	78.6	113.1	132.7	201.1	314.2	490.9	615.8	804.4	1,256.8
$N_{Rk,s}$ (kN) Characteristic Yield	23.1	36.1	52.0	61.1	92.5	144.5	225.8	283.3	370.0	578.1
$N_{Rd,s}$ (kN) Design Yield	20.1	31.4	45.2	53.1	80.4	125.7	196.4	246.3	321.7	502.7

Setting Time before applying load

Ambient temperature (°C)	Max time for installation (min)	Waiting time before applying load (hr)
32°C	8.5	2
27°C	12	2
20°C	15	2
16°C	18	3
10°C	21	6

MATERIAL
Grade 460 steel

INSTALLATION



Chemical Resistance of EPCON G5 Anchor

Chemical substances	Resistance	Chemical substances	Resistance
Xylene	1	Toluene	2
Gasoline	1	10% Nitric Acid	2
20% Caustic NaOH (Sodium Hydroxide)	1	8.5% Ammonium Hydroxide	2
Fresh water	1	5% Bleach	3
Salt Water	1	Acetone	3
10% Sulfuric Acid (H ₂ SO ₄)	2	Glacial Acetic Acid	3
3.5% Hydrochloric Acid (HCL)	2	Methanol	3
9% Phosphoric Acid	2	Methylene Chloride	3

- 1 = High resistance (Anchors could be submerged in these materials)
- 2 = Medium resistance (Anchors could be temporary submerged due to splash or spill)
- 3 = Low resistance (Anchors should be limited to splash and spill followed by immediate cleanup)

EPCON G5

Rebar (FE460)



2/8

Number of Anchors per cartridge

Rebar diameter	10	12	13	16	20	25	28	32	40
Drilling Ø (mm)	13	15	16	20	25	30	35	40	50
Drilling depth (mm)	90	110	110	125	170	210	270	300	400
No. of anchors per cartridge									
EPCON G5 (650ml)	108.8	66.9	58.8	33.1	15.6	8.8	5.0	3.4	1.7

Ultimate Loads ($N_{Ru,m}$, $V_{Ru,m}$) / Characteristic Loads (N_{Rk} , V_{Rk}) in kN

TENSILE @ Concrete strength 30 N/mm²

Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	90	110	110	125
$N_{Ru,m}$ (kN)	25.0	39.0	56.2	65.9	99.9
N_{Rk} (kN)	23.1	36.1	52.0	61.1	92.5

SHEAR @ Concrete strength 30 N/mm²

Rebar size	T8	T12	T13	T16	T20
$V_{Ru,m}$ (kN)	15.0	23.4	33.7	39.6	59.9
V_{Rk} (kN)	13.9	21.7	31.2	36.6	55.5

Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
$N_{Ru,m}$ (kN)	156.1	243.9	305.9	399.6	624.4
N_{Rk} (kN)	144.5	225.8	283.3	370.0	578.1

Rebar size	T20	T25	T28	T32	T40
$V_{Ru,m}$ (kN)	93.7	146.3	183.6	239.8	374.6
V_{Rk} (kN)	86.7	135.5	170.0	222.0	346.9

Design Loads (N_{Rd} , V_{Rd}) for one anchor without edge or spacing influence in kN

$$N_{Rd} = \frac{N_{Rk}}{\gamma_{Mc,N}}$$

$$V_{Rd} = \frac{V_{Rk}}{\gamma_{Ms,V}}$$

TENSILE @ Concrete strength 30 N/mm²

Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	110	110	110	125
N_{Rd} (kN)	15.4	24.1	34.7	40.7	61.7

SHEAR @ Concrete strength 30 N/mm²

Rebar size	T8	T10	T12	T13	T16
V_{Rd} (kN)	11.1	17.3	25.0	29.3	44.4

Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
N_{Rd} (kN)	96.4	150.6	188.9	246.7	385.4

Rebar size	T25	T25	T28	T32	T40
V_{Rd} (kN)	69.4	108.4	136.0	177.6	277.5

$\gamma_{Ms,N} = 1.5$ (steel failure)

$\gamma_{Ms,V} = 1.25$

Recommended Loads (N_{rec} , V_{rec}) for one anchor without edge or spacing influence in kN

$$N_{rec} = \frac{N_{Rk}}{\gamma_{Mc,N} \cdot \gamma_F}$$

$$V_{rec} = \frac{V_{Rk}}{\gamma_{Ms,V} \cdot \gamma_F}$$

TENSILE @ Concrete strength 30 N/mm²

Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	90	110	110	125
N_{rec} (kN)	11.0	17.2	24.8	29.1	44.0

SHEAR @ Concrete strength 30 N/mm²

Rebar size	T8	T10	T12	T13	T16
V_{rec} (kN)	7.9	12.4	17.8	20.9	31.7

Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
N_{rec} (kN)	68.8	107.5	134.9	176.2	275.3

Rebar size	T20	T25	T28	T32	T40
V_{rec} (kN)	49.6	77.4	97.1	126.9	198.2

$\gamma_F = 1.4$

$\gamma_{Ms,N} = 1.5$ (steel failure)

$\gamma_F = 1.4$

$\gamma_{Ms,V} = 1.25$

steel failure

EPCON G5

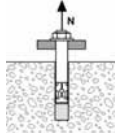
Rebar (FE460)



3/8

CC-Method

TENSILE in kN



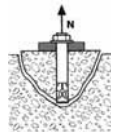
Pull-out resistance
Concrete strength 30 N/mm²

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_B \cdot f_T$$

$N_{Rd,p}^0$	Design pull-out resistance				
Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	90	110	110	125
$N_{Rd,p}^0$ (kN)	14.8	20.8	30.5	33.0	46.2

$N_{Rd,p}^0$	Design pull-out resistance				
Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
$N_{Rd,p}^0$ (kN)	78.5	121.2	174.5	188.1	278.8

$$\gamma_{Mc,N} = 1.8$$



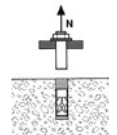
Concrete cone resistance
Concrete strength 30 N/mm²

$$N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_T \cdot \Psi_s \cdot \Psi_{c,N}$$

$N_{Rd,c}^0$	Design cone resistance				
Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	90	110	110	125
$N_{Rd,c}^0$ (kN)	26.3	31.4	42.5	42.5	51.4

$N_{Rd,c}^0$	Design cone resistance				
Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
$N_{Rd,c}^0$ (kN)	81.6	112.0	163.3	191.3	294.5

$$\gamma_{Mc,N} = 1.5$$



Steel resistance

$N_{Rd,s}$	Steel design tensile resistance				
Rebar size	T8	T10	T12	T13	T16
$N_{Rd,s}$ (kN)	15.4	24.1	34.7	40.7	61.7

$N_{Rd,s}$	Steel design tensile resistance				
Rebar size	T20	T25	T28	T32	T40
$N_{Rd,s}$ (kN)	96.4	150.6	188.9	246.7	385.4

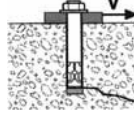
$$\gamma_{Ms,N} = 1.5$$

$$N_{Rd} = \min(N_{Rd,p}; N_{Rd,c}; N_{Rd,s})$$

$$\beta_N = N_{Sd} / N_{Rd} \leq 1$$

$$\beta_N + \beta_V \leq 1.2$$

SHEAR in kN



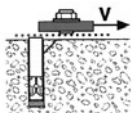
Concrete edge resistance
Concrete strength 30 N/mm²

$$V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_{\beta,V} \cdot \Psi_{s-c,V}$$

$V_{Rd,c}^0$	Design concrete edge resistance at a minimum edge distance (c_{min})				
Rebar size	T8	T10	T12	T13	T16
h_{ef} (mm)	80	90	110	110	125
c_{min}	40	45	55	55	63
s_{min}	40	45	55	55	63
$V_{Rd,c}^0$ (kN)	2.6	3.4	5.1	5.2	6.9

$V_{Rd,c}^0$	Design concrete edge resistance at a minimum edge distance (c_{min})				
Rebar size	T20	T25	T28	T32	T40
h_{ef} (mm)	170	210	270	300	400
c_{min}	85	105	135	150	200
s_{min}	85	105	135	150	200
$V_{Rd,c}^0$ (kN)	12.4	18.9	30.0	37.4	65.2

$$\gamma_{Mc,V} = 1.5$$

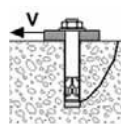


Steel resistance

$V_{Rd,s}$	Steel design shear resistance				
Rebar size	T8	T10	T12	T13	T16
$V_{Rd,s}$ (kN)	11.1	17.3	25.0	29.3	44.4

$V_{Rd,s}$	Steel design shear resistance				
Rebar size	T20	T25	T28	T32	T40
$V_{Rd,s}$ (kN)	69.4	108.4	136.0	177.6	277.5

$$\gamma_{Ms,V} = 1.25$$



Concrete pry-out failure
Concrete Strength 30 N/mm²

$$V_{Rd,cp} = V_{Rd,cp}^0 \cdot f_B \cdot \Psi_s \cdot \Psi_{c,N}$$

$V_{Rd,cp}^0$	Design pry-out resistance				
Rebar size	T8	T10	T12	T13	T16
$V_{Rd,cp}^0$ (kN)	52.7	62.9	84.9	84.9	102.9

$V_{Rd,cp}^0$	Design pry-out resistance				
Rebar size	T20	T25	T28	T32	T40
$V_{Rd,cp}^0$ (kN)	163.2	224.0	326.6	382.5	588.9

$$\gamma_{Mc,V} = 1.5$$

$$V_{Rd} = \min(V_{Rd,c}; V_{Rd,s}; V_{Rd,cp})$$

$$\beta_V = V_{Sd} / V_{Rd} \leq 1$$

f_B INFLUENCE OF CONCRETE

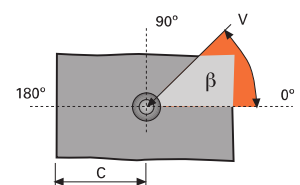
Concrete Grade	f_B	Concrete Grade	f_B
C16/20	0.81	C35/45	1.21
C20/25	0.90	C40/50	1.28
C25/30	1.00	C45/55	1.34
C30/37	1.10	C50/60	1.40

f_T INFLUENCE OF EMBEDMENT DEPTH

$$f_T = \frac{h_{act}}{h_{ef}} \quad \text{where: } h_{ef} \leq h_{act} \leq 2h_{ef}$$

$f_{\beta,V}$ INFLUENCE OF SHEAR LOADING DIRECTION

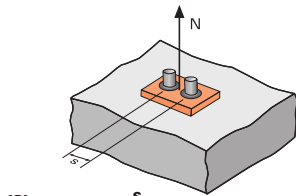
Angle β [°]	$f_{\beta,V}$
0~50	1.0
60	1.1
70	1.2
80	1.5
90~180	2.0





CC-Method

Ψ_s INFLUENCE OF SPACING FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD



$$\Psi_s = 0.5 + \frac{s}{4h_{ef}}$$

$$s_{min} < s < s_{cr,N}$$

$$s_{min} = 0.5h_{ef}$$

$$s_{cr,N} = 2h_{ef}$$

Ψ_s must be used for each spacing influenced the anchors group

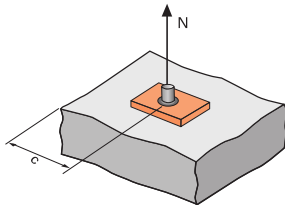
Spacing, s Reduction Factor Ψ_s
Non-cracked concrete

	T8	T10	T12	T13	T16
40	0.63				
45	0.64	0.63			
55	0.67	0.65	0.63	0.63	
65	0.70	0.68	0.65	0.65	0.63
85	0.77	0.74	0.69	0.69	0.67
105	0.83	0.79	0.74	0.74	0.71
140	0.94	0.89	0.82	0.82	0.78
160	1.00	0.94	0.86	0.86	0.82
180		1.00	0.91	0.91	0.86
220			1.00	1.00	0.94
250					1.00

Spacing, s Reduction Factor Ψ_s
Non-cracked concrete

	T20	T25	T28	T32	T40
85	0.63				
105	0.65	0.63			
140	0.71	0.67	0.63		
160	0.74	0.69	0.65	0.63	
210	0.81	0.75	0.69	0.68	0.63
250	0.87	0.80	0.73	0.71	0.66
300	0.94	0.86	0.78	0.75	0.69
350	1.00	0.92	0.82	0.79	0.72
420		1.00	0.89	0.85	0.76
540			1.00	0.95	0.84
600				1.00	0.88
700					0.94
800					1.00

$\Psi_{c,N}$ INFLUENCE OF EDGE FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD



$$\Psi_{c,N} = 0.275 + 0.725 \cdot \frac{c}{h_{ef}}$$

$$c_{min} < c < c_{cr,N}$$

$$c_{min} = 0.5h_{ef}$$

$$c_{cr,N} = h_{ef}$$

$\Psi_{c,N}$ must be used for each distance influenced the anchors group

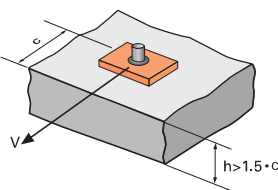
Edge, c Reduction Factor $\Psi_{c,N}$
Non-cracked concrete

	T8	T10	T12	T13	T16
40	0.63				
45	0.68	0.63			
55	0.77	0.71	0.63	0.63	
63	0.84	0.78	0.69	0.69	
80	1.00	0.91	0.80	0.80	
85		0.95	0.83	0.83	0.63
90		1.00	0.86	0.86	0.65
110			1.00	1.00	0.74
125					0.80
170					1.00

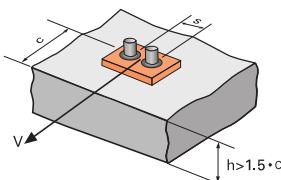
Edge, c Reduction Factor $\Psi_{c,N}$
Non-cracked concrete

	T20	T25	T28	T32	T40
85	0.63				
105	0.72	0.63			
135	0.85	0.74	0.63		
150	0.91	0.79	0.67	0.63	
170	1.00	0.86	0.73	0.68	
200		0.96	0.81	0.75	0.63
210		1.00	0.83	0.78	0.65
270			1.00	0.92	0.76
300				1.00	0.81
400					1.00

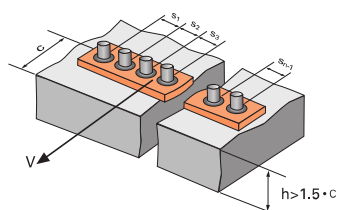
$\Psi_{s-c,V}$ INFLUENCED OF SPACING AND EDGE DISTANCE FOR CONCRETE EDGE RESISTANCE IN SHEAR LOAD



$$\Psi_{s-c,V} = \frac{c}{c_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$



$$\Psi_{s-c,V} = \frac{3c + s}{6c_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$



FOR SINGLE ANCHOR FASTENING

Reduction Factor $\Psi_{s-c,V}$
Non-cracked concrete

$\frac{c}{c_{min}}$	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2
$\Psi_{s-c,V}$	1.00	1.31	1.66	2.02	2.41	2.83	3.26	3.72	4.19	4.69	5.20	5.72

FOR 2 ANCHORS FASTENING

Reduction Factor $\Psi_{s-c,V}$
Non-cracked concrete

$\frac{c}{c_{min}}$	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2
$\frac{s}{c_{min}}$												
1.0	0.67	0.84	1.03	1.22	1.43	1.65	1.88	2.12	2.36	2.62	2.89	3.16
1.5	0.75	0.93	1.12	1.33	1.54	1.77	2.00	2.25	2.50	2.76	3.03	3.31
2.0	0.83	1.02	1.22	1.43	1.65	1.89	2.12	2.38	2.63	2.90	3.18	3.46
2.5	0.92	1.11	1.32	1.54	1.77	2.00	2.25	2.50	2.77	3.04	3.32	3.61
3.0	1.00	1.20	1.42	1.64	1.88	2.12	2.37	2.63	2.90	3.18	3.46	3.76
3.5		1.30	1.52	1.75	1.99	2.24	2.50	2.76	3.04	3.32	3.61	3.91
4.0			1.62	1.86	2.10	2.36	2.62	2.89	3.17	3.46	3.75	4.05
4.5				1.96	2.21	2.47	2.74	3.02	3.31	3.60	3.90	4.20
5.0					2.33	2.59	2.87	3.15	3.44	3.74	4.04	4.35
5.5						2.71	2.99	3.28	3.57	3.88	4.19	4.50
6.0							2.83	3.11	3.41	3.71	4.02	4.33

FOR OTHER CASE OF FASTENINGS

$$\Psi_{s-c,V} = \frac{3c + s_1 + s_2 + s_3 + \dots + s_{n-1}}{3nc_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$

EPCON G5

Rebar (FE460)



5/8

Installation in G30 Reinforced Concrete

Design Embedment Depth $L_{b,rqd}$ and Design Tensile Load Table N_{Rd}

Rebar Ø (mm)	10	12	13	16	20	25	28	32	40
Hole Ø (mm)	13-14	15-16	16-18	20-22	25-28	30-32	35-38	40-42	50-52
Design Yield, N_{Rd} (kN)	31.4	45.2	53.1	80.4	125.7	196.3	246.3	321.7	502.7
$L_{b,rqd}$ (mm)	140	165	180	220	275	340	385	515	725
$n = L_{b,rqd} / \text{Rebar } \varnothing$	14	14	14	14	14	14	14	17	19
Min Edge Distance (mm), $L_{b,min}$	50	60	65	80	100	125	140	160	200
Min Spacing Distance (mm), $L_{b,min}$	100	120	130	160	200	250	280	320	400
L_b (mm)	N_{Rd} (kN)								
100	23.1								
110	25.4								
120	27.7	33.2							
125	28.9	34.6							
130	30.0	36.0	39.0						
140	32.3	38.8	42.9						
145		40.2	43.5						
160		44.3	48.0	59.1					
165		45.7	49.5	61.0					
180			54.0	66.5					
190				70.2					
200				73.9	92.4				
205				75.7	94.7				
220				81.3	101.6				
250					115.4	144.3			
255					117.8	147.2			
275					127.0	158.7			
280						161.6	181.0		
315						181.8	203.6		
320						184.7	206.9	200.6	
340						196.3	219.8	213.2	
360							232.7	225.7	
385							248.9	241.4	
395								247.6	
400								250.8	278.8
440								275.9	306.7
485								304.1	338.1
515								322.9	359.0
570									397.4
595									414.8
650									453.1
725									505.4

Safety Factor for bond $\gamma_B = 1.8$

Safety Factor for Concrete $\gamma_{Mc,N} = 1.5$

Safety Factor for Steel $\gamma_{Ms,N} = 1.15$

Tensile development length L_b using Epcon G5:

where the $F_{Rd} \leq N_{Rd,s}$:

$$L_b = \left(\frac{L_{b,rqd}}{f_B} \right) \cdot \left(\frac{F_{Rd}}{N_{Rd,s}} \right)$$

f_B INFLUENCE OF CONCRETE

Concrete Grade	f_B	Concrete Grade	f_B
C16/20	0.81	C35/45	1.21
C20/25	0.90	C40/50	1.28
C25/30	1.00	C45/55	1.34
C30/37	1.10	C50/60	1.40

Note: For splitting and splice calculation, please refer to ITW Technical Engineers.

EPCON G5

Rebar (FE460)



6/8

Installation in Reinforced Concrete

EXAMPLE 1:

The design action effect which causes tension in the starter bar is:

$$N = 650\text{kN/m run}$$

Strip footing details:

Concrete grade = 25N/mm²

Structure Thickness = 600mm

Concrete cover = 50mm

Load case induced in starter bar = 650kN/m run

Consider design of 460N/mm² grade reinforcement bar

To satisfy Strength Limit State Design Criteria,

$$\begin{aligned} \text{therefore, } N &\leq A_s \cdot \frac{f_{yk}}{\gamma_{Ms}} \\ 650,000(\text{N}) &\leq A_s \cdot (460 \div 1.15) \\ A_s &\geq 1,624\text{mm}^2 \end{aligned}$$

Using 4T25 reinforcing bar @ 300mm c/c = 1,963.6mm² > 1,624mm²

Installing T25 with Epcon G5:

$$L_b = \left(\frac{L_{b,rd}}{f_b} \right) \cdot \left(\frac{F_{Rd}}{N_{Rd,s}} \right)$$

$$L_b = (340\text{mm} \div 0.9) \times (162.5\text{kN} \div 196.3\text{kN})$$

$$L_b = 312.7\text{mm} \dots \text{say } 315\text{mm}$$

EXAMPLE 2:

where the existing structure is 380mm deep and concrete cover remains 50mm:

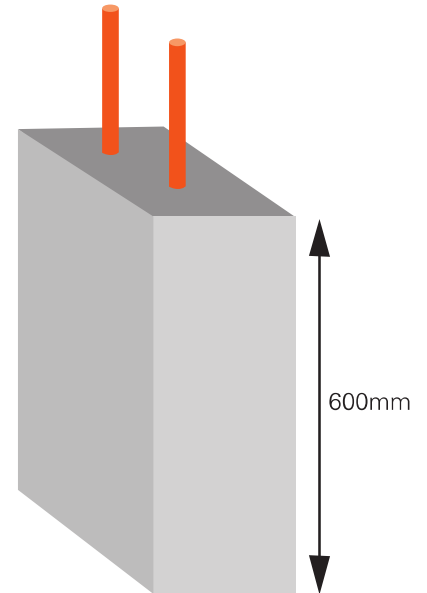
hole depth = 330mm

design tensile capacity for T25 @ 300mm embedment depth = 173.1kN x 0.9 = 155.7kN

650kN = n x 155.7kN

n = 650kN / 155.7kN = 4.17 ~ 5 (round to nearest number)

Use 5T25 reinforcing bar @ 225mm c/c = 2,454.5mm² > 1,624mm²



CHEMICAL ANCHORS

EPCON G5

Rebar (FE460)



7/8

Rebar Connection Design as per EN 1992-1-1

General points

The design of rebar connections and determination of the internal section forces to be transferred in the construction joint shall be in keeping with the EN 1992-1-1.

Verification of immediate local force transfer to the concrete has been provided.

Verification of the transfer of the loads to the anchored to the building component must be provided.

Connection joint

In case of a connection being made between new and existing concrete where the surface layer of the existing concrete is carbonated, the layer should be removed in the area of the new reinforcing bar (with a diameter $d_s + 60\text{mm}$) prior to the installation of the new bar. The forgoing may be neglected if building components are new and not carbonated.

To prevent damage of the concrete during drilling, the following requirements has to be met:

- Minimum concrete cover:
 $c_{\min} = 30 + 0.06l_v \geq 2d_s$ (mm) for hammer drilled holes
where l_v = actual embedment depth
- Minimum distances between 2 rebars:
 $s = 40\text{mm} \geq 4d_s$
- Minimum embedment:
 $l_{b,\min} = 1.5 \cdot \max(0.3 \cdot L_{bd}; 10\emptyset; 100\text{mm})$

Furthermore, the minimum concrete cover according to EN 1992-1-1 SS 4.4.1.2 must be observed.



Rebar Application Under ETA Rule - Intended Use

Overlap Joint

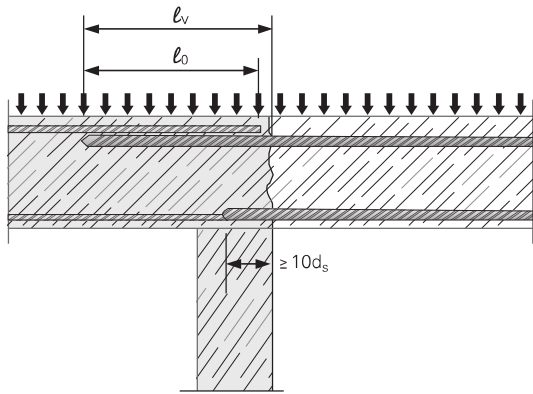


Figure 1.1: Overlap joint for rebar connections of slabs and beams.

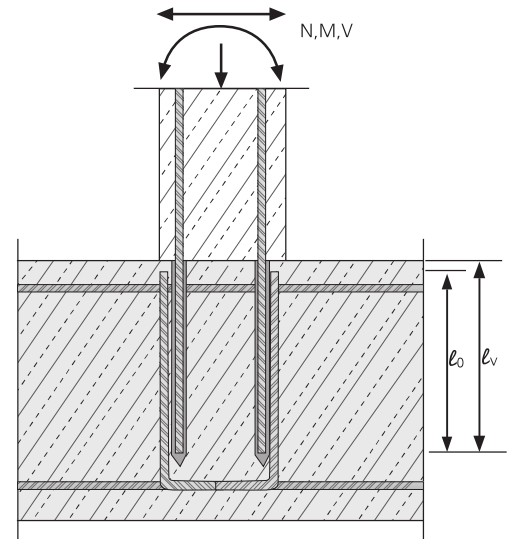


Figure 1.2: Overlap joint at a foundation of a column or wall where the rebars are stressed in tension

Anchoring Bar

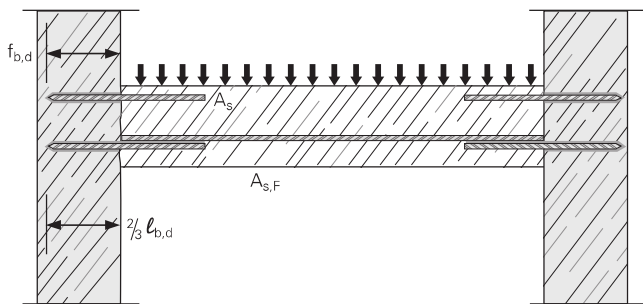


Figure 1.3: End anchoring of slabs or beams design as simply supported.

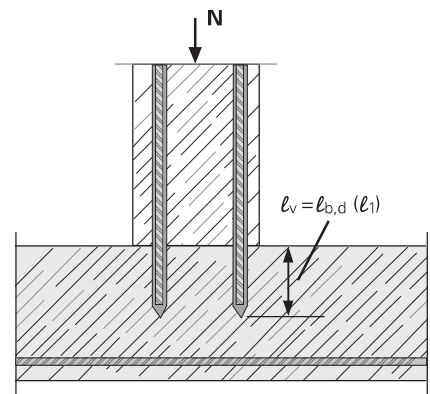


Figure 1.4: Rebar connection for components stressed primarily in compression. The rebars are stressed in compression.

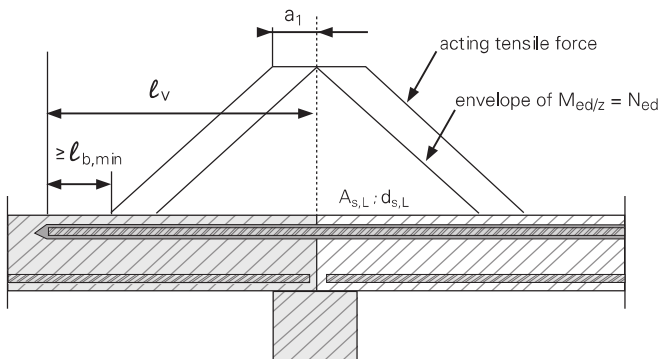


Figure 1.5: Anchoring of reinforcement to cover the line of acting tensile force.

EPCON G5

Zinc Coated Anchor Stud (G5.8)



1/4

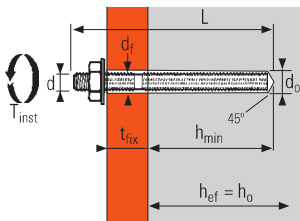
Epoxy Resin - High Performance



CHEMICAL ANCHORS



ICC-ESEVALUATION REPORT
ESR-1137



Technical Data

EPCON G5 with Spit Stud	Anchor depth (mm)	Max thick of fixture (mm)	Drill depth (mm)	Min thick of base material (mm)	Ø Thread (mm)	Ø Drill bit (mm)	Total anchor length (mm)	Max tighten torque (Nm)	Spit stud code	Ramset power tool code	Drill bit type-size
	$h_{ef,min}$	t_{fix}	h_0	h_{min}	d	d_0	L	T_{inst}			
M8	80	15	80	100	8	10	110	10	060215	DD527	R3 PLUS-10
M10	90	20	90	115	10	12	130	20	060216	DD527	R3 PLUS-12
M12	110	25	110	140	12	14	160	30	060217	DD527	R3 PLUS-14
M16	125	35	125	160	16	18	190	60	060218	DD543	R3 PLUS-18
M20	170	65	170	215	20	25	260	120	060219	DD565	R3 MAX-25
M24	210	63	210	270	24	28	300	200	060220	DD565	R3 MAX-28
M30	280	70	280	350	30	35	380	400	060221	DD565	R3 MAX-35

EPOXY G5 Two part cartridge, 100% epoxy resin - vol. 650ml

Anchor Mechanical Properties

Anchor size	M8	M10	M12	M16	M20	M24	M30
As (mm ²) Stressed cross-section	36,6	58,0	84,3	157,0	227,0	330,0	530,0
W _{el} (mm ²) Elastic section modulus	31,2	62,3	109,2	277,5	482,4	845,5	1721,0
Zinc coated steel stud, grade 5.8							
f _{uk} (N/mm ²) Min. tensile strength	520	520	520	520	520	520	520
f _{yk} (N/mm ²) Yield strength	420	420	420	420	420	420	420
M ⁰ _{rk,s} (Nm) Characteristic bending moment	19,5	39,0	68,0	173,0	301,0	528,0	1074,0
M (Nm) Recommended bending moment	8,0	15,9	27,8	70,7	122,9	215,4	438,3

Setting Time before applying load

Ambient temperature (°C)	Max time for installation (min)	Waiting time before applying load (hr)
32°C	8,5	2
27°C	12	2
20°C	15	2
16°C	18	3
10°C	21	6

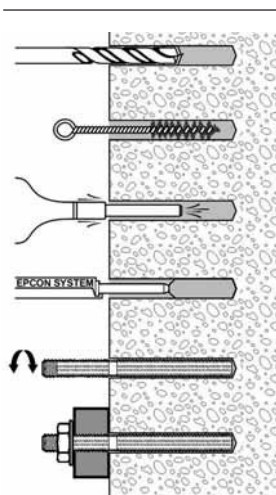
Chemical Resistance of EPCON G5 Anchor

Chemical substances	Resistance	Chemical substances	Resistance
Xylene	1	Toluene	2
Gasoline	1	10% Nitric Acid	2
20% Caustic NaOH (Sodium Hydroxide)	1	8.5% Ammonium Hydroxide	2
Fresh Water	1	5% Bleach	3
Salt Water	1	Acetone	3
10% Sulfuric Acid (H ₂ SO ₄)	2	Glacial Acetic Acid	3
3.5% Hydrochloric Acid (HCL)	2	Methanol	3
9% Phosphoric Acid	2	Methylene Chloride	3

- 1 = High resistance (Anchors could be submerged in these materials)
- 2 = Medium resistance (Anchors could be temporary submerged due to splash or spill)
- 3 = Low resistance (Anchors should be limited to splash and spill followed by immediate cleanup)

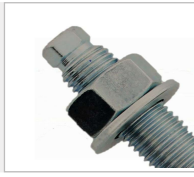
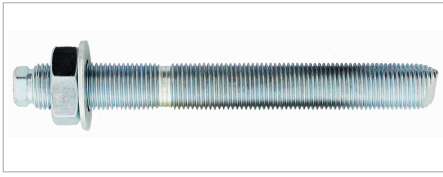
MATERIAL

Zinc coated steel version :
Stud M8-M30 :
Steel grade 5.8 as per ISO 898-1
Nut : Steel grade 6 or 8
NF EN 20898-2
Washer : Steel DIN 513
Protection : zinc coated 5 µm min.
NF E25-009



ANCHOR TECHNOLOGY

ChemSet™ Spit Studs-ZincPlated



Part No.	Description	Anchor Size (mm)	Stud Length (mm)	Max Fixture Thickness (mm)	Hole Diameter Ø (mm)	Min Hole Depth (mm)	Order Qty
8A-STUD-0811-0S	SPIT STUD M 8x110	8	110	15	10	80	10
8A-STUD-1013-0S	SPIT STUD M10x130	10	130	20	12	90	10
8A-STUD-1216-0S	SPIT STUD M12x160	12	160	25	14	110	10
8A-STUD-1619-0S	SPIT STUD M16x190	16	190	35	18	125	10
8A-STUD-2026-0S	SPIT STUD M20x260	20	260	65	25	170	10
8A-STUD-2430-0S	SPIT STUD M24x300	24	300	63	28	210	10
8A-STUD-3038-0S	SPIT STUD M30x380	30	380	70	35	280	5

ChemSet™ Anchor Studs - Stainless Steel SUS316



Part No.	Description	Anchor Size (mm)	Stud Length (mm)	Max Fixture Thickness (mm)	Hole Diameter Ø (mm)	Min Hole Depth (mm)	Order Qty
7D-CS08-110S-S	CS08110SS Studs	8	110	15	10	80	10
7D-CS10-130S-S	CS10130SS Studs	10	130	20	12	90	10
7D-CS12-160S-S	CS12160SS Studs	12	160	25	14	110	10
7D-CS16-190S-S	CS16190SS Studs	16	190	35	18	125	10
7D-CS20-260S-S	CS20260SS Studs	20	260	65	25	170	6
7D-CS24-300S-S	CS24300SS Studs	24	300	63	28	210	6
7D-CS30-380S-S	CS30380SS Studs	30	380	70	35	280	5

ChemSet™ Anchor Studs - Hot Dipped Galvanised



Part No.	Description	Anchor Size (mm)	Stud Length (mm)	Max Fixture Thickness (mm)	Hole Diameter Ø (mm)	Min Hole Depth (mm)	Order Qty
8A-CS08-110H-DG	CS08110GH Studs	8	110	15	10	80	10
8A-CS10-130H-DG	CS10130GH Studs	10	130	20	12	90	10
8A-CS12-160H-DG	CS12160GH Studs	12	160	25	14	110	10
8A-CS16-190H-DG	CS16190GH Studs	16	190	35	18	125	10
8A-CS20-260H-DG	CS20260GH Studs	20	260	65	25	170	6
8A-CS24-300H-DG	CS24300GH Studs	24	300	63	28	210	6

EPCON G5

Zinc Coated Anchor Stud (G5.8)



2/4

Number of Anchors per cartridge

Stud diameter	8	10	12	16	20	24	30
Drilling Ø (mm)	10	12	14	18	25	28	35
Drilling depth (mm)	80	90	110	125	170	210	280
No. of anchors per cartridge							
EPCON G5 (650ml)	206.9	127.7	76.8	40.9	15.6	10.1	4.8

Ultimate Loads ($N_{Ru,m}$, $V_{Ru,m}$) / Characteristic Loads (N_{Rk} , V_{Rk}) in kN

TENSILE @ Concrete strength 30 N/mm ²								SHEAR @ Concrete strength 30 N/mm ²							
Anchor size	M8	M10	M12	M16	M20	M24	M30	Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef} (mm)	80	90	110	125	170	210	280	$V_{Ru,m}$ (kN)	12.8	20.3	29.5	52.9	82.6	118.9	176.2
$N_{Ru,m}$ (kN)	21.3	33.8	49.2	88.2	137.6	198.2	293.6	V_{Rk} (kN)	11.9	18.8	27.3	49.0	76.4	110.1	163.1
N_{Rk} (kN)	19.8	31.3	45.5	81.6	127.4	183.6	271.9								

Design Loads (N_{Rd} , V_{Rd}) for one anchor without edge or spacing influence in kN

$$N_{Rd} = \frac{N_{Rk}}{\gamma_{Mc,N}}$$

$$V_{Rd} = \frac{V_{Rk}}{\gamma_{Ms,V}}$$

TENSILE @ Concrete strength 30 N/mm ²								SHEAR @ Concrete strength 30 N/mm ²							
Anchor size	M8	M10	M12	M16	M20	M24	M30	Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef} (mm)	80	90	110	125	170	210	280	V_{Rd} (kN)	9.5	15.0	21.9	39.2	61.2	88.1	130.5
N_{Rd} (kN)	13.2	20.9	30.3	54.4	84.9	122.4	181.2	$\gamma_{Ms,V} = 1.25$							
$\gamma_{Ms,N} = 1.5$ (steel failure)															

Recommended Loads (N_{rec} , V_{rec}) for one anchor without edge or spacing influence in kN

$$N_{rec} = \frac{N_{Rk}}{\gamma_{Mc,N} \cdot \gamma_F}$$

$$V_{rec} = \frac{V_{Rk}}{\gamma_{Ms,V} \cdot \gamma_F}$$

TENSILE @ Concrete strength 30 N/mm ²								SHEAR @ Concrete strength 30 N/mm ²							
Anchor size	M8	M10	M12	M16	M20	M24	M30	Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef} (mm)	80	90	110	125	170	210	280	V_{rec} (kN)	6.8	10.7	15.6	28.0	43.7	62.9	93.2
N_{rec} (kN)	9.4	14.9	21.7	38.9	60.7	87.4	129.5	$\gamma_F = 1.4$							
$\gamma_F = 1.4$								$\gamma_{Ms,V} = 1.25$							
$\gamma_{Ms,N} = 1.5$ (steel failure)															

steel failure

EPCON G5

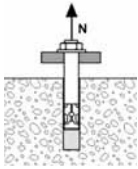
Zinc Coated Anchor Stud (G5.8)



3/4

CC-Method

TENSILE in kN

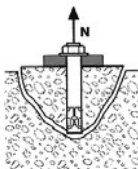


Pull-out resistance
Concrete strength 30 N/mm²

$$N_{Rd,p} = N^0_{Rd,p} \cdot f_B \cdot f_T$$

$N^0_{Rd,p}$	Design pull-out resistance						
Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef}	80	90	110	125	170	210	280
$N^0_{Rd,p}$ (kN)	15.3	26.7	33.8	45.5	59.1	90.1	150.1

$$\gamma_{Mc,N} = 1.8$$

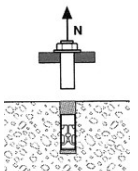


Concrete cone resistance
Concrete strength 30 N/mm²

$$N_{Rd,c} = N^0_{Rd,c} \cdot f_B \cdot f_T \cdot \Psi_s \cdot \Psi_{c,N}$$

$N^0_{Rd,c}$	Design cone resistance						
Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef} (mm)	80	90	110	125	170	210	280
$N^0_{Rd,c}$ (kN)	26.3	31.4	42.5	51.4	81.6	112.0	172.5

$$\gamma_{Mc,N} = 1.5$$



Steel resistance

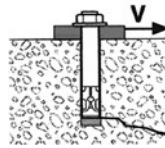
$N_{Rd,s}$	Steel design tensile resistance						
Anchor size	M8	M10	M12	M16	M20	M24	M30
$N_{Rd,s}$ (kN)	13.2	20.9	30.3	54.4	84.9	122.4	181.2

$$\gamma_{Ms,N} = 1.5$$

$$N_{Rd} = \min(N_{Rd,p}; N_{Rd,c}; N_{Rd,s})$$

$$\beta N = N_{Sd} / N_{Rd} \leq 1$$

SHEAR in kN

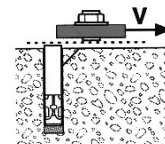


Concrete edge resistance
Concrete strength 30 N/mm²

$$V_{Rd,c} = V^0_{Rd,c} \cdot f_B \cdot f_{\beta,V} \cdot \Psi_{s-c,V}$$

$V^0_{Rd,c}$	Design concrete edge resistance at a minimum edge distance (c_{min})						
Anchor size	M8	M10	M12	M16	M20	M24	M30
h_{ef} (mm)	80	90	110	125	170	210	280
c_{min}	40	45	55	65	85	105	140
s_{min}	40	45	55	65	85	105	140
$V^0_{Rd,c}$ (kN)	2.6	3.4	5.1	7.3	12.4	18.7	32.6

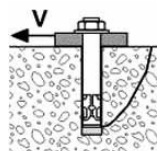
$$\gamma_{Mc,V} = 1.5$$



Steel resistance

$V_{Rd,s}$	Steel design shear resistance						
Anchor size	M8	M10	M12	M16	M20	M24	M30
$V_{Rd,s}$ (kN)	9.5	15.0	21.9	39.2	61.2	88.1	130.5

$$\gamma_{Ms,V} = 1.25$$



Concrete pry-out failure
Concrete Strength 30 N/mm²

$$V_{Rd,cp} = V^0_{Rd,cp} \cdot f_B \cdot \Psi_s \cdot \Psi_{c,N}$$

$V^0_{Rd,cp}$	Design pry-out resistance						
Anchor size	M8	M10	M12	M16	M20	M24	M30
$V^0_{Rd,cp}$ (kN)	52.7	62.9	84.9	102.9	163.2	224.0	344.9

$$\gamma_{Mc,V} = 1.5$$

$$V_{Rd} = \min(V_{Rd,c}; V_{Rd,s}; V_{Rd,cp})$$

$$\beta V = V_{Sd} / V_{Rd} \leq 1$$

$$\beta N + \beta V \leq 1.2$$

f_B INFLUENCE OF CONCRETE

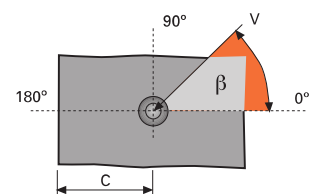
Concrete Grade	f_B	Concrete Grade	f_B
C16/20	0.81	C35/45	1.21
C20/25	0.90	C40/50	1.28
C25/30	1.00	C45/55	1.34
C30/37	1.10	C50/60	1.40

f_T INFLUENCE OF EMBEDMENT DEPTH

$$f_T = \frac{h_{act}}{h_{ef}} \quad \text{where: } h_{ef} \leq h_{act} \leq 2h_{ef}$$

$f_{\beta,V}$ INFLUENCE OF SHEAR LOADING DIRECTION

Angle β [°]	$f_{\beta,V}$
0~50	1.0
60	1.1
70	1.2
80	1.5
90~180	2.0



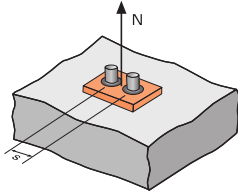
EPCON G5

Zinc Coated Anchor Stud (G5.8)



CC-Method

Ψ_s INFLUENCE OF SPACING FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD



$$\Psi_s = 0.5 + \frac{s}{4h_{ef}}$$

$$s_{min} < s < s_{cr,N}$$

$$s_{min} = 0.5h_{ef}$$

$$s_{cr,N} = 2h_{ef}$$

Ψ_s must be used for each spacing influenced the anchors group

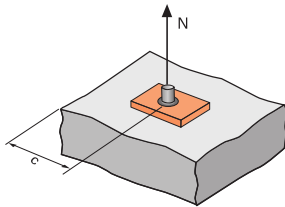
Spacing, s Reduction Factor Ψ_s
Non-cracked concrete

	M8	M10	M12	M16
40	0.63			
45	0.64	0.63		
55	0.67	0.65	0.63	
65	0.70	0.68	0.65	0.63
85	0.77	0.74	0.69	0.67
105	0.83	0.79	0.74	0.71
140	0.94	0.89	0.82	0.78
160	1.00	0.94	0.86	0.82
180		1.00	0.91	0.86
220			1.00	0.94
250				1.00

Spacing, s Reduction Factor Ψ_s
Non-cracked concrete

	M20	M24	M30
85	0.63		
105	0.65	0.63	
140	0.71	0.67	0.63
160	0.74	0.69	0.64
180	0.76	0.71	0.66
220	0.82	0.76	0.70
250	0.87	0.80	0.72
300	0.94	0.86	0.77
340	1.00	0.90	0.80
370		0.94	0.83
420		1.00	0.88
560			1.00

$\Psi_{c,N}$ INFLUENCE OF EDGE FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD



$$\Psi_{c,N} = 0.275 + 0.725 \cdot \frac{c}{h_{ef}}$$

$$c_{min} < c < c_{cr,N}$$

$$c_{min} = 0.5h_{ef}$$

$$c_{cr,N} = h_{ef}$$

$\Psi_{c,N}$ must be used for each distance influenced the anchors group

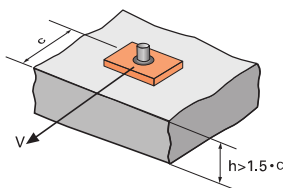
Edge, c Reduction Factor $\Psi_{c,N}$
Non-cracked concrete

	M8	M10	M12	M16
40	0.63			
45	0.68	0.63		
55	0.77	0.71	0.63	
65	0.86	0.79	0.70	0.65
80	1.00	0.91	0.80	0.73
90		1.00	0.86	0.79
110			1.00	0.91
125				1.00

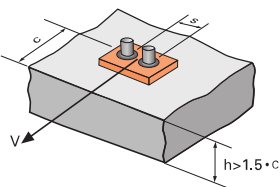
Edge, c Reduction Factor $\Psi_{c,N}$
Non-cracked concrete

	M20	M24	M30
85	0.63		
105	0.72	0.63	
120	0.78	0.68	
140	0.87	0.75	0.63
170	1.00	0.86	0.71
210		1.00	0.81
250			0.92
280			1.00

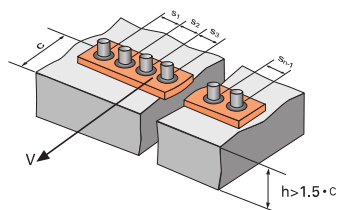
$\Psi_{s-c,V}$ INFLUENCED OF SPACING AND EDGE DISTANCE FOR CONCRETE EDGE RESISTANCE IN SHEAR LOAD



$$\Psi_{s-c,V} = \frac{c}{c_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$



$$\Psi_{s-c,V} = \frac{3c + s}{6c_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$



FOR SINGLE ANCHOR FASTENING

Reduction Factor $\Psi_{s-c,V}$
Non-cracked concrete

$\frac{c}{c_{min}}$	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2
$\Psi_{s-c,V}$	1.00	1.31	1.66	2.02	2.41	2.83	3.26	3.72	4.19	4.69	5.20	5.72

FOR 2 ANCHORS FASTENING

Reduction Factor $\Psi_{s-c,V}$
Non-cracked concrete

$\frac{c}{c_{min}}$	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2
$\frac{s}{c_{min}}$												
1.0	0.67	0.84	1.03	1.22	1.43	1.65	1.88	2.12	2.36	2.62	2.89	3.16
1.5	0.75	0.93	1.12	1.33	1.54	1.77	2.00	2.25	2.50	2.76	3.03	3.31
2.0	0.83	1.02	1.22	1.43	1.65	1.89	2.12	2.38	2.63	2.90	3.18	3.46
2.5	0.92	1.11	1.32	1.54	1.77	2.00	2.25	2.50	2.77	3.04	3.32	3.61
3.0	1.00	1.20	1.42	1.64	1.88	2.12	2.37	2.63	2.90	3.18	3.46	3.76
3.5		1.30	1.52	1.75	1.99	2.24	2.50	2.76	3.04	3.32	3.61	3.91
4.0			1.62	1.86	2.10	2.36	2.62	2.89	3.17	3.46	3.75	4.05
4.5				1.96	2.21	2.47	2.74	3.02	3.31	3.60	3.90	4.20
5.0					2.33	2.59	2.87	3.15	3.44	3.74	4.04	4.35
5.5						2.71	2.99	3.28	3.57	3.88	4.19	4.50
6.0							2.83	3.11	3.41	3.71	4.02	4.33

FOR OTHER CASE OF FASTENINGS

$$\Psi_{s-c,V} = \frac{3c + s_1 + s_2 + s_3 + \dots + s_{n-1}}{3nc_{min}} \cdot \sqrt{\frac{c}{c_{min}}}$$